Construction and Quality Assurance – Quality Control High Capacity Micropiles in Mined Ground for Bridge Suppor Route 249/171

Joplin, Missouri

International Society for Micropiles 8<sup>th</sup> International Workshop on Micropiles Toronto, Ontario, Canada

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### ELEMENTS OF CONSTRUCTION RISK MANAGEMENT

- Issue a GBR Geotechnical Baseline Report all claims would be based on the GBR.
- Prequalification Process Owner and Engineer Reviews Contractor Qualifications
- Real time Construction Monitoring
- Regular Team Meetings to Resolve Problems
  - Monthly Progress Meetings All Parties
  - Weekly Construction Meetings between Resident Engineer and Contractor



#### **Contractor Prequalification**

#### WHY?

- Site extremely complex with highly variable geology
- Methods proposed to investigate and treat the foundation rock would require interactive implementation during construction
- The ground conditions required a high level of construction expertise, flexibility, and responsiveness.
- The contractor would need to be able to react to variations in drilling and grouting conditions and highly variable quantities between adjacent hole locations



### **Contractor Prequalification Requirements**

- Corporate
  - 5 similar projects in last 3 years
- Personnel
  - Superintendent with 5 years of similar job experience
- Specific Technical Experience
  - Drilling and grouting in similar conditions
  - Monitor While Drilling Experience
  - Ability to process, interpret, display and act upon data during construction
- Installation and Work Plan
  - Safety plan with special consideration to mine shafts
  - Operation sequence
  - Drilling Methods
  - Grout and reinforcement placement
  - Details of automated recording and reporting systems
  - Details of grout types
  - Details of micropile testing equipment and procedures
  - Understanding the scope of work and interaction with investigation, pretreatment, and practical operational interface with the Engineer.



### **CRITICAL STEPS OF CONSTRUCTION INSPECTION**

CHARACTERIZE THE GROUND

-Site geology and subsurface conditions were deemed "chaotic" -Subsurface conditions could change dramatically from boreholes drilled less than a meter apart

#### **CONSTRUCTION ENGINEERING AND INSPECTION**

Log each hole with subsurface conditions
Verify ground conditions matched design assumptions
Change design values based on conditions encountered

\*Do all this with minimal delay



### **Real Time Monitoring During Construction**

- Due to unpredictable nature of site, each treatment hole would be used to characterize the ground.
- Full time inspectors on each rig will log each hole.
- Contractor will furnish Monitor While Drilling (MWD) parameters to inspectors after each hole.
- Inspectors will interpret subsurface and prescribe treatment in real time.
- Avoid delays Resolve conflicts. Resident Engineer charged with resolving most conflicts with contractor.



#### Drill Rig Controls with Monitor While Drilling Instrumentation Drilling Rate- Thrust Pressure - Torque





# Manual Monitoring

Rig Inspectors Recorded:

- o penetration rate
- o lithology
- o flush return characteristics (cuttings, volume)
- o drill "action"
- o interconnections between holes
- o hole stability
- o groundwater observations



#### **Inspectors Drill Log**

#### HNTB

**DRILLING LOG** 

Route 249 - Jasper County, MO

MoDOT Project No. J7U0436K HNTB Job No 42500-CN-001-001

Start Date and Time:	10-12-2006	Drill Rig Type/# BAPTOR / IR 750
		Casing and Hole Ø 130 m
Hole Number	49 87-84	Inclination of Hole 90*
Driiler's Name	DEVID BEWITT	Drilling system
Elevation at 0.0 depth	1	Page / of 2
Logged By	SATH PAINTER	

Start	ime Finish	From	h (m) To	Penetr. Rate	Material Description Provide description min every 1meter .	Remarks
09:12	09:18	0	0.73		LMESTONE - LITTLE NEATH	HZO ON.
09:17	09:24	0.73	1.73	•.	LIMESTONE - SOLID	HID ON INT M.
09:24	09:31	1.73	3.81		EMESTONE/TA CROKEN/WEATH.	HZU UN INTM.
09:36	09:36	3.3/	4.81		LIMESTONE - LITTLE BROKEN	ADD ROD / HOD OFF
69:41	09:47	4.81	7.95		LIMESTONE	Had DEF
09:47	09:54	7.85	10.84	· · ·	LIMESTONE-SOLIO/OUSTY	H2 0 ON ADD ROD
10010	10:08	NO.84	13.60		LIMESTONE - NEATHERED	GROUND Had
10:08	10:11	13.60	14.32		BACKEN/WEATH LIMESTONE	FEW CORPLES - CHEAT
10:11	10:13	14.52	16.81		CHERT / Y LIMESTONE	ADD ROD
10:18	10:25	16.89	20.16		LIMESTONE SUME CHERT / CLAY	STGADY GROCAD H20
10:25	10:29	20.16	21.85		LIMESTONE/SOME CHERT/ CLAY	STEADY ENOUND HED
10:29	10:30	21.85	22.97		CLAY/ ADCH - PEN HITS	400 ROP )
10; 34	10:34	22.97	23.15		LITTLE ROCK - HATING	
10:34	10:37	23.15	27.80		CLAY/ TA- ROCK - FEW HUTS	CUAY CHAT
10:37	10:38	27.80	28.52		VERY BROKEN ROCK-HITS-1	RUSH THRO
10:38	10:39	28-52	29.05		CLAYTA POCK - FBW HITS	ADD ROD
10:45	10:48		38.80		VERY BOSHEN POCH	· · · · · · · · · · · · · · · · · · ·
1.1.1		30-80	31.05		UNY TA ROCH - FEW HI	13
10548	10:48	31.05	3672		VERY BROKON ROCKA	
10:48	10:39		35.14		CLAY / ROCK-FEWHITS	REAM ADD ROD
Note: Indicate	e bottom of cas	sing depth a	ind final dri	I depth.	-	

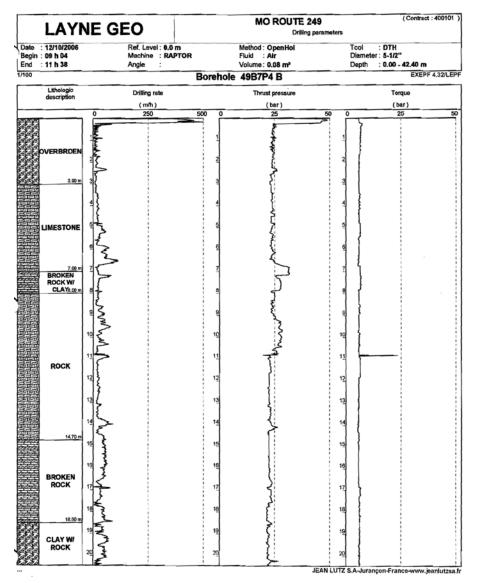
Contractor Representative:

Owner Representatives

HN 

-LEAKS IN HZD LINE MAY BE FEEDING HOLE INDEPENDANT OF SWITCH

#### **Contractors MWD Drill Parameter Log**





#### **Inspectors Low Mobility Grout Log**

# 

Job#:		·		- `		ole: <u>49 37 - '</u>				
			10-19-06	<del>/</del>	Pu	mp Type: جرب	wing WP500	×		
Casing Casing	Diamete		-		Slump: 4 Mes					
Depth	Time		Maximum Gage	Grout Quantity		Reason for Stopping	Comments			
(meters)	Start	Stop	Pressure (psi)	Strokes	Cubic Feet	Injection				
50	14:00	14:05	500	90	11.1	1	Truck / Slon	44		
50	14 03	-14:05	500	45		1	1			
48	14:08	1410	600	104		1	-Remark Rd	]		
AL.	14.0	14:35	200	378	0,	4	1 8461	8 4 4 2		
46	14:45	1210	300	393	1.0	4	Truck 2	16yds		
44	15:10	15:40	250	375	121	4	Truch3 5	lum \$4.5" 24		
44	15.45	16:10	400	385		4	Truck 4	lum p 4.5" 24y 32 yds		
46	1610	16.35	300	.388		4	Truck 5.	40yd.		
46	1640	17:00	300	272		Rod Schissory	Truck 6			
44	11:00	17:08	350	358		.4 *		Heyds Brit Par Remove Boc		
44	-17:00	17.10	600	lele			Truck 7	Briller		
42	17:18	17:30	250	395		4	; slevel	Remove Col		
42	17:30	1735	500	130		1	Truck 8	1.1		
60	1222	nus	490	335		· · · · · · · · · · · · · · · · · · ·	Rot siesing			
30	11:45	1700	400	380		4	64vds	69xels 10-18		
Resta	sccl ob	ained	36m	depth				10-19-06		
36	3.50	255	Sies	72			Truck 1 Sh	mp 31/2"		
34	\$155	2010	400			1				
32	B5 G	8157	600	85			0			
30	8:04	805	600+			· /	Remove Roa			
28	8:05	801	400	10		/	70.14			
26	8:06	8:46	300	405		4	72 Yds			
26	9:45	900	606 <sup>4</sup>	110	:		Truck 2			
24	900	9:01	400	112		Grout Leak, 4 = S		_Revoue Ros		

Total Volume Injected Durin	ng This Operation	:_ 84	cubic vard	64 on 10-1300
Owner Representative:				200,10-17.00
Contractor Representative:				

HNTB

#### **MWD Low Mobility Grout Log**

M	O F	۲۲	24	9	LAYNE GEOCONST (Contract N# : 400101 COMPACTION GROUTING
					Pressure/Volume/Flow
Prookie	pressure	0	bar		<pre><site settings=""></site></pre>
Safety p			0 bar		Sleeve volume : 0 I Delay time : 0 s (current set of instructions)
ouloty p	1035010		o bai		Borehole 49B7P4 10/17/06
Top levei (m)	Bottom level (m)	Grout	Number of regrouts	Injected volume (I)	Final pressure() Average pressure()     Cumulated volume     Average flow       Steeve N#     (bar)     (0)     (/min)       0     10     20     30     40     0     375     750     1125     15000     0     112.5     225     337.5     44
0.00	2.44	1	1	70.0	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
2.44	4.44	1	1	40.0	20 40.0556770 20 40.0556770 20
4.44	6.44	1	1	50.0	19 50.0556070) 19 50.0556070) 19
6.44	8.44	1	1	90.0	18 800 18.00000 18 90.000099970) 18
8.44	10.44	1	1	40.0	17 40.0659070) 17 40.0659070) 17
10.44	12.44	1	1	60.0	16 00 00 16 00 00 16 00 00 00 00 00 00 00 00 00 00 00 00 00
12.44	14.44	1	1	190.0	15 1200000 15 100.00000000 15
14.44	16.44	1	1	90.0	14 8.0005557014
16.44	18.44	1	1	140.0	13 28.400) 13 140.000070) 13
18.44	20.44	1	1	130.0	12 130.0859970) 12 130.0859970) 12
20.44	22.44	1	1	16320.0	11 17.0800 11
22,44	24.44	1	1	8830.0	10 15 0 10 10 10 10 10 10 10 10 10 10 10 10 1
24.44	26.44	1	1	4300.0	9 21.0ptio) 9 ///////////////////////////////////
26.44	26.44	1	1	2490.0	8
28.44	30.44	1	1	80.0	7
30,44	32.44	1	1	160.0	6 15.0(00) 6 180.0(59970) 6
32.44	34.44	1	1	150.0	5 5 19.00(99970) 5 190.00(99970) 5
34.44	36.44	1	1	60.0	4 21.5 50) 4 90.0 29970) 4
36.44	36.44	1	1	28840.0	3 22060) 3 ///////////////////////////////////
38.44	40.44	1	1	6480.0	2 13760) 2 ///////////////////////////////////
40.44	42.44	1	1	340.0	1 10,7(60) 1 340,0(59970) 1

Cumulated volume (Gr.)	: 68970		Number of grouted sleeve(s): 21
Sampling (Gr.)	:28	(*) : measured values	(*) : requested values



#### **Inspectors High Mobility Grout Log**



Job#:	: Rou	te 249 -	– Jasper		]	Hole: _ 4	_ of 966-5-9
Date:	10-	10-06		Grout O	peration:		
Date H	lole W	as Dril	lled: 10	-11-06			
Stage Depth Interval (Meters)	Time	Pressure (PSI)	Injection Rate (Gal/Min)	Total Volume Reading (Gallons)	Cumulative Take For Stage (Gallons)	Reason For Stopping Injection	Comments and QA/QC Data Marsh Cone, Specific Gravity
15	13:14	0	0	0			MixA
	13:15	189	14 32	13			Air Pressure 110 ps
	13:16	0	0	32		Minitren	I not working
	13117	0	0	.32			
	13:18	1120	18 45	46			
	13:19	0	0	55	· ·	Pump mal	Sundian
	13,50	0	0	55			5.6.=153 : M.C=45
	13:25	٥ د د ا	20.46	147		check-Pa	iser
	13:29.	0	0	147			
	13:20	1730	1922	170			
	13:35	3340	209-	274			
	13:40	40-50	20 29	386		CheckPac	ther replace NI Bottle
	13:48	0	0	386			A is Province HSpai
	13:49	3432	13 -	403			
	13:50		172'	413			MIS
	13:55	6940	12.40	495			56167 MEG4
		8760	024	505			Start Rebusal
Y	14:01	872	009	505			

Reason for Stoppage: I = Refusal Pressure, 2 = Ground Heave, 3 = Grout Leak, 4 = Shift End/Breakdown/Grout Supply

505

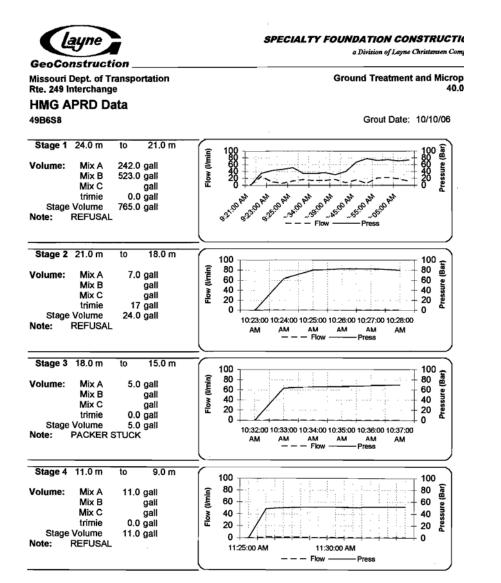
14:02 8750 009

21

Total Volume Injected During This Operation: 39179-15=>1980 ch **Owner Representative:** Contractor Representative:



#### **MWD High Mobility Grout Log**





#### **Inspectors Micropile Installation Log**

	HN	TB			554		
	Project Name:	Route 249 - Jasp	er Co.	-06 Bar 11-19-06			
	Location/Area:	Bridge 6	149	L, 6	Bent6	Design Capacity 1565	N
$\cap$	Weather/Tomp		_	<u></u>		Pile No.: 4986 -	MP-67
time and the second			tandar	d Mi	cropile installation Log a	ind Report (Use Additional Pages a	n Needod)
	struction Dimensions						(B) Reinforcing Steel Specifications
Date D	Standard Drilling & Drive L rilled (Complete):	11-19-0	6		Final Stick-Out		Casing Diameter 193.7 mm x 12.7 mm
Surface	E: 296.80	(Bottom of Footin	ng)		Casing	0.25 m	Bar Diameter: 63.5 mm - Epoxy Coated
II				~			Comments:
Depth (nim)	- Daw	Ground Profile		2000	Pie insulation	Connection Description:	
0.00	MORE TO COLORING THE		225783	ð	CENTRALIZER	337mm × 114mm 00	[
1 000	Duerb	010	Þ	ч	COUTLER	9.4) Total Length (m)	
1		ant	50	I	_	Top of Casing (m)	(C) Cesing Grouting
2 000		tř –	5	H	LENTRALIZER	-O- Top Re-Steel (m)	Date of Casing Grouting: 11-16-06
3 000	V Vie		4A	ľ		9.4] Bottm Re-Steel (m)	Cement Type: IT Supplier: Ashgrove (B)
-					Coupler	Depth of Re-Steel mm (Approx.) Approx. Location of Centralizers:	W/C Ratio: 0.45 Spec. Gravity: 1.95 (Weasured with barold mud balance
4 000		estone.	3			Approx. Location of Centralizers:	Admixtures/Dosage: Grouting Method: Tremie Tube / cap and pressure
5 000		1. S	V.C.H		LENTRALIZER	245/160 Hole Diameter (mm)	Grouting Method: <u>Tremie Tube / cap.and.pressure</u> (Le Tremie Tube) Comments
			de l	7		72.9 Hole inclination, Deg. Verified by Angle Tool	Pressure: Volume: 0.16 m <sup>3</sup>
6 000			F	- 15		30/100 Batter	
	Limes	onew/grout	S		-	Commonts/Remarks:	QA/QC; [].e. cubies, barnid balance, march cone, ASTM, PTI, elg
7 000		9/	ndi				(D) Bond Zone Grouting
		er.	No.		CENTRALIZER		Date of Bond Grouting_11-19-06
( ) =	'ine	,	9	•	_		Cement Type I - I Supplier: Ashgrove (
9 000			H١	ł	<u> </u>		W/C Ratio: 0,45 Spec. Gravity: 1.94
10 000				╢			Admixtures/Dosage:
=			Ħ				1
11 000			日			-	Pressure: Volume: 0.21 m <sup>3</sup>
Ξ			$\exists$		-	QAVQC Testing:	OAQ2: (Le. colves, bend balleges, vanish cont, ASTM (FT), etc)
12 000			비				
13 000			曰				·
=	1 1		비		-		
14 000							Additional Remarks/notes:
15 000			Ħ		_		Additional Remarks/notes: Cased Length 3.20 m
=			E		-		Bonded Length 6.50 M
16 000	: H		비				Total Length 9.30 m
	1 E		E		-	Depth of Scale	
17 000		_	Ħ			Top of Rock Bottom of Hole	
18 000	1 E		E			Bottom of Hote Bottom of Casing	
=			Ħ		-	Location of Casing Joints	
19 000						Locations of Centralizers	
20 000			Ħ			Location of ReGrout Tubes Extent of Bond Zone	
=	1 日		日			Exert of Bond Kone	
<u> </u>			Ħ			-	
<b>-</b> ─				_			1
	J. Duehnin					11-20-06	Lavrie's Representative Dete:
				_		L-010	Capito a registrativa de Carto.



#### **Four Phases of Route 249 Construction**

#### • Mine Shaft Closure

- Explore at suspected shaft locations
- Close known shaft locations and those found during exploration
  - Type 1 Closure Fill with LMG
  - Type 2 Closure Cap with Structural Concrete
- Ground Treatment
  - Explore and Fill Mine Voids at Bridge Approach Embankments and at known mined areas near bridge foundations
  - Involved Systematic drilling and treating with LMG on a grid pattern to a predetermined depth
- Foundation Treatment
  - Excavate Foundation Locations and Examine for Mining Activity
  - Treat with Combination of LMG and HMG to Improve Ground and Limit Micropile Grout Take
- Micropiles
  - Install and Test Design Verification Piles
  - Install Production Micropiles
  - Perform Proof Tests



#### Mine Shaft Closure – Type 1 "Critical Shaft"

• Definition:

"Mine shafts/open features located <u>within</u> 15-meters of a major structural element or <u>within</u> 5-meters of the footprint of an embankment or cut."

- Construction Procedure
  - Exploratory Mine Feature Inspection Excavation Dig with Backhoe
  - If Mine Feature Exists:
  - Excavate to top of rock
  - Drill and grout from base of shaft to top of rock
  - Verify closure with two secondary holes, grout as necessary



#### Abandoned Mine Shaft - 1.5 x 1.5 meters 45 meters deep





#### **Grouting Mine Shaft with Low Mobility Grout**





#### Injection of Low Mobility Grout into Mine Shaft Previously Capped by MDNR





### Placement of Low Mobility Grout with Concrete Pump





#### Mine Shaft Closure – Type 2 "Non-Critical Shaft"

• Definition:

"Mine shafts/open features located <u>beyond</u> 15-meters of a major structural element or <u>beyond</u> 5-meters of the footprint of an embankment or cut."

- Construction Procedure
  - Exploratory Mine Feature Inspection Excavation Dig with Backhoe

If Mine Feature Exists:

- Excavate to top of rock
- Plug throat with polyurethane foam and cap with concrete



#### **Expose Mine Shaft Opening**





#### Type 2 Mine Shaft Closure – Structural Concrete Cap





#### Ground Treatment – Bridges – Walls - Culverts

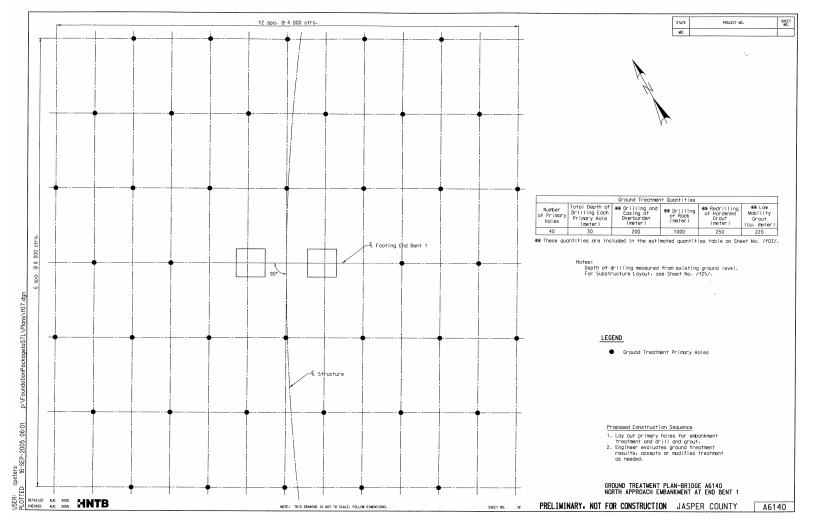
• Philosophy:

Locate and treat unforeseen mine features and/or voids

- Construction Procedure
  - Drill primary holes (30-meters deep) in predetermined pattern shown on drawings
  - Grout primary holes utilizing Low Mobility Grout (LMG)
    - Small fissure grouting is not necessary



#### Typical Layout - Ground Treatment - 4 Meter Grid



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#### **Typical Hole Layout for Ground Treatment**





# Ground Treatment at Retaining Wall





#### **Ground Treatment at Box Culverts**





#### "Making Water" Adjacent Drill Hole Foundation Treatment





### "Losing Water" Adjacent Drill Hole





### Controlling Artesian Groundwater with High Mobility Grout





### Spread Footings Bearing on Rock





#### **Micropiles - Installing Permanent Casing**





#### **Drilling Battered Micropile Bond Length**





#### Attaching Tremie Tube and Centralizers





### Micropile Reinforcing – 2 $\frac{1}{4}$ " Thread bars





#### **Installing Reinforcing Bars**





### **Mixing Micropile Grout**





#### **Freshly Grouted Micropile**





#### **Micropile with Nut and Shear Plate**





## **Six Piles for Typical Footing**





#### **Construction Costs "K" Job**

Planned Cost = \$5,396,238

Actual Cost = \$5,565,100

3 Percent over run of plan cost

**Fun Facts** 

•17,895 Meters of Drilling = 58,710 Feet = 11 Miles

•6964 Cubic Meters of Grout = 9108 Cubic Yards = 2 Miles of Two Lane Roadway

•3,270 Meters Micropiles = 10,728 Feet = 2 Miles



# Foundation Package Design Team

Geotechnical, Structural Engineering and Construction Engineering and Inspection

HNTB

Micropiles and Grouting

Geosystems Inc – Donald Bruce Isherwood and Associates – Marcelo Chuaqui

**Rock Mechanics** 

Wyllie and Norrish – Norm Norrish

